Methodological Notes on the Regional Level Validation of a Microscopic Traffic Simulation Model

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ABSTRACT

Traffic simulation models have been increasingly used to evaluate and compare alternative complex realworld traffic problems. Simulation is safer, less expensive and faster than field testing. The past few years have witnessed substantial development of transportation network modeling tools and stronger emphasis on addressing the need to model large-scale networks more accurately and efficiently. While these simulation models can be helpful to transportation engineers, the models must be well calibrated and validated before they can provide credible results. However, simulation models have been often conducted under default parameters. This is mainly due to either the difficulties in field data collection or the lack of knowledge of the appropriate procedure to calibrate and validate traffic simulation models.

This paper presents the results of a recent effort to microscopically simulate the regional evacuation plan for New Orleans Metropolitan Area during the hurricane Katrina. The model involved over 300,000 vehicles moving within a road network that covered several thousand square miles over a 48 hour period. Output statistics were generated on a second-by-second basis for each traveler in the system. Model validation was based upon a comparison of the TRANSIMS generated traffic volumes to the corresponding traffic volumes actually observed during the 2005 hurricane Katrina evacuation. The validation process included the percent error estimation and the regression analysis between the simulated and observed traffic volume data. This study was unique in that it is among the first to develop validation criteria for a regional model based on actual traffic data collected during a live regional mass evacuation.

Analysis was performed utilizing percent errors estimation based on direct comparisons of the hourly volumes at each counting station. Also, an alternative validation approach was carried out using regression analysis between the cumulative observed and simulated volumes for the same stations by analyzing the fit for the regression line $y = a + bx + \varepsilon$. The error percentage and the fit were found to be reasonable with an error percentage less than 25 percent and an R-squared value of over 0.80. This indicated that the TRANSIMS simulation model was a realistic representation of the evacuation operations observed during the hurricane Katrina.

KEYWORDS: Validation, Regional validation, Microscopic simulation, Regional simulation, TRANSIMS.

INTRODUCTION

Traffic simulation models are increasingly used to evaluate and compare alternative complex real-world

traffic problems. Early studies seeking to apply to traffic simulation models were limited in their geographical scales and time durations (Theodoulou and Wolshon, 2004; Kwon and Pitt, 2005; Jha et al., 2004). Recent simulation models such as TRANSIMS,

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DynaSmart, VISSIM, DynusT and CORSIM, have the ability to model second-by-second movements of hundreds of thousands of individual vehicles, moving over vast geographic areas, for periods as long as several days. It is well known that even the most detailed input into the most detailed simulation models has the potential to yield unrealistic or even useless results. Thus, a critical requirement in the development of any simulation model is the validation of the output results. Validation helps to insure, or at least demonstrate, the level of accuracy, so that the output results of the model are reasonably close to those of the essence of the actual system that is being modeled. A validated model also gives a base point from which it is possible to make changes and assess modifications to the system. In such an arrangement, the theory is that once a model is able to reproduce the desired essence reality in the base case, then any different outcomes that result from modifications to the system can logically be assumed to be a consequence of the changes.



Figure 1: Logical Diagram for Model Validation Source: (Law and Kelton, 1991)

To date, the review of the state-of-the-art traffic simulation models reveals that many studies addressing the calibration and validation process of microscopic and macroscopic simulation models were limited in their geographical scales and time durations. Kunde (2002) used the speed-density relationship and capacity to calibrate DynaMIT-P. Kim and Rilett (2003) used the simplex algorithm to optimize the degree-of-fit for their models in CORSIM and TRANSIMS. Ma and Abdulhai (2002) and Lee et al.



Figure 2: Coding methodology Source: (TRANSIMS Open Source)

(2001) used PARAMICS traffic simulation model to determine values for the calibration parameters of mean target headway and mean reaction time. Recently, Chiu et al. (2008) conducted a regional scale traffic simulation model using DynusT for the Houston-Galveston area during hurricane Rita, to evaluate various evacuation scenarios, but due to the lack of available data the simulation output could not be calibrated to Rita evacuation. Unfortunately, the calibration and validation of a microscopic regional traffic simulation model output are extremely challenging due to the large geographical area, the detailed network in addition to the long simulation duration. Model calibration and validation form naturally a statistical process in which the uncertainty due to data and model errors should be accounted for. Nearly any statistical test would reject the results of models at this level, even those that were reasonably accurate. Therefore, the choice of the statistical test used to compare the observed and simulated values is a critical task. The conceptual framework for the validation methodology is described in Figure 1. The key question in Figure 1 is: "Is the model valid/do model results realistically represent reality"? The statistical techniques provide a quantified answer to this question. According to Rouphail and Sacks (2003), the probability that the difference between the observed output and the simulated output is less than a specified tolerable difference within a given level of significance can be written as:

P { |observed-simulated output | \leq d} > α



Figure 3: Location of LA DOTD count stations used for volume comparison

where:

- d: tolerable difference threshold indicating how close the model is to reality;
- α : significance level that tells how the results are obtained from the simulation model.

The key methodological steps for building a valid and credible simulation model are (Law and Kelton, 1991):

- Verification: which is concerned with building the model correctly to ensure the model performance,
- Validation: which is concerned with the accuracy

of the model and

- Credibility of the model: which is concerned with the acceptance of the model by the user.

Balci (1998) defines a successful simulation model to be "the one that produces a sufficiently credible solution that is accepted by decision makers". This involves the assessment of the simulation model quality throughout the verification and validation of the simulation models.

In this paper, system validation was based upon a comparison of the TRANSIMS generated traffic

volumes to the corresponding traffic volumes actually observed during the 2005 hurricane Katrina evacuation in New Orleans Metropolitan Area. The validation process incorporated a number of steps leading up to quantitative comparisons of the data sets to evaluate the results. The following sections summarize the various data sources and methods used in the validation process as well as the results gained from them.

		Westbound		Eastbound	Northbound	Southbound		
	Time	Station 54	Station 27	Station 18	MDOT	Station15	Station 88	
	(hours after	I-10	US-61	US-190	Station I-59	I-55	US-90	Total
	midnight 8/2//05)	Westbound	Westbound	Westbound	Northbound	Northbound	Southbound	
		433	146	82	306	224	140	1,331
	1	323	102	69	202	155	115	966
	2	217	81	58	151	98	117	722
	3	235	57	41	149	93	90	665
	4	206	130	55	171	79	78	719
	5	350	127	109	208	136	174	1,104
	6	502	183	283	230	230	208	1,636
	7	693	225	428	338	384	230	2,298
	8	950	234	567	559	499	356	3,165
	9	1,317	326	726	793	651	519	4,332
	10	1,838	374	784	1,062	853	602	5,513
	11	1,816	571	819	1,143	1,158	749	6,256
	12	1,743	881	716	1,059	1,196	982	6,577
	13	1,704	1,342	731	1,271	1,498	1,201	7,747
	14	1,630	1,686	663	1,418	1,616	1,652	8,665
	15	1,064	1,785	761	1,112	2,121	1,792	8,635
	16	1,446	1,675	893	1,168	2,148	2,095	9,425
	17	2,412	1,743	940	1,526	2,001	2,313	10,935
	18	2,174	1,670	970	1,694	2,395	1,994	10,897
	19	1,815	1,565	1,022	1,200	2,451	1,771	9,824
	20	1,939	1,279	929	612	2,537	2,119	9,415
	21	1,901	583	819	532	2,215	1,272	7,322
	22	1,805	544	923	438	1,474	991	6,175
	23	1,795	515	070	434	1,032	778	5,222
pc	24	1,701	490	342	222	922	750	4,495
eric	25	1,797	412	202	202	1,100	615	4,700
v P	20	1,778	413	101	272	1.028	015	4,231
lov	27	2 349	927	255	485	1,028	1 1 0 0	4,930 6 780
raf	28	2,349	1 344	566	700	1,303	2,006	8 594
ont	30	2,134	1,344	730	1 138	2 599	1,692	10 415
Ŭ	31	2,525	1 881	1.009	1,150	3,280	1,02	12,141
	32	2,505	1,804	1,009	1,109	4 017	2,309	13.314
	33	2,493	1.760	1,259	1.943	4.407	2.325	14,187
	34	2.554	1.695	1.400	1.887	4.660	2.037	14.233
	35	2,442	1.660	1.352	2.134	4,742	2.052	14,382
	36	2,574	1,708	1,343	2,212	4,833	2,112	14,782
	37	2,504	1,696	1,372	2,043	4,710	2,057	14,382
	38	2,353	1,684	1,313	1,789	4,893	2,035	14,067
	39	2,477	1,680	1,404	1,609	4,695	2,114	13,979
	40	2,210	1,733	1,300	2,303	4,600	2,146	14,292
	41	1,432	1,540	1,373	3,009	3,951	2,139	13,444
	42	573	816	1,228	2,097	2,766	2,177	9,657
	43	275	52	673	1,901	1,888	2,214	7,003
	44	163	22	305	682	531	2,162	3,865
	45	119	17	217	64	255	1,876	2,548
	46	81	7	130	28	183	1,336	1,765
	47	54	6	75	8	118	272	533

Table 1. LA DOTD data station observed evacuation volume

	T .	Westbound			Eastbound	astbound Northbound Southbound		
	Time	Station 54	Station 27	Station 18	MDOT	Station15	Station 88	T (1
	(nours after	I-10	US-61	US-190	Station I-59	I-55	US-90	Total
	munight 8/27/05)	Westbound	Westbound	Westbound	Northbound	Northbound	Southbound	
		256	14		83	24	99	476
	1	463	12		163	355	210	1,203
	2	332	9		106	286	162	895
	3	258	11		104	189	140	702
	4	252	8		101	192	146	699
	5	378	8		128	187	166	867
	6	532	15		220	326	241	1,334
	7	681	29		320	491	407	1,928
	8	1,085	25		379	609	476	2,574
	9	1,583	35		559	906	710	3,793
	10	2,010	53		705	1,229	944	4,941
	11	2,395	58		918	1,495	1,083	5,949
	12	2,658	78		940	1,749	1,276	6,701
	13	2,895	87		1,085	1,890	1,369	7,326
	14	3,056	102		1,229	2,016	1,496	7,899
	15	3,127	144		1,335	2,410	1,573	8,589
	16	2,448	775		1,351	2,716	1,609	8,899
	17	3,706	108		1,431	2,577	1,875	9,697
	18	3,646	726	3	1,645	2,682	1,993	10,695
	19	3,591	1,178	11	1,472	3,419	2,112	11,783
	20	2,905	1,103	55	1,390	2,961	1,988	10,402
	21	2,816	719	15	1,219	2,636	1,661	9,066
	22	2,534	358	1	980	2,010	1,360	7,243
	23	2,322	245	1	871	1,727	1,091	6,257
ъ	24	2,090	86		731	1,542	942	5,391
rio	25	1,949	79	5	664	1,256	943	4,896
Pe	26	2,005	61	1	652	1,347	866	4,932
MO	27	1,955	53	3	680	1,241	850	4,782
afl	28	2,401	54	2	735	1,348	1,107	5,647
ntı	29	3,123	65	6	1,032	1,943	1,510	7,679
Co	30	3,552	100	5	1,299	2,441	1,676	9,073
	31	3,863	206	1	1,575	2,601	1,902	10,154
	32	4,050	1,233	4	1,824	2,856	2,186	12,153
	33 24	4,057	2,039	10	2,044	4,442	2,420	15,012
	54 25	5,085	2,271	1	2,002	4,227	2,473	14,121
	33 26	2,927	2,289		2,121	3,942	2,505	13,/84
	30 27	2,921	2,181		2,088	3,923	2,303	13,470
	38	2,937	2,231		2,085	3,972	2,330	13,575
	30	2,942	2,200		2,041	3,973	2,433	13,055
	40	2,810	2,232		2,007	4 045	2,451	12 605
	40	2 664	2,575		2,200	3 743	2,377	13 527
	42	3 014	2,373	4	1 772	3 534	2,402	12,578
	43	2 908	731	+ 5	1 342	2 952	2,224	10.059
	44	2,275	200	19	911	2,080	1,993	7.478
	45	1.417	49	2	539	1.122	1,964	5.093
	46	925	18	-	341	737	1.075	3.096
I	47	491	7		169	486	226	1,379
F	Total	107 157	31 948	160	51 797	98.616	67 728	357 406

Table 2. TRANSIMS simulated evacuation traffic volume

Time Interval	General Travel Direction	Location	Observed Traffic Volume (vph)	Simulated Traffic Volume (vph)	Volume Difference (vph)	Error %	Error % by Time Increment
		I-10 Westbound	15,021	21,961	6,940	46.20	
3		US 61 Westbound	8,250	688	-7,562	-91.66	
to flo	West	US 190	6,892	N/A	-6,892	-100.00	-7.38
-15 ior tra		Westbound					
0 Pri	East	I-59 Northbound	10,172	8,375	-1,797	-17.67	
C	North	I-55 Northbound	10,991	14,354	3,363	-30.60	
	South	US 90 Southbound	9,005	10,948	1,493	16.58	
w		I-10 Westbound	52,138	70,635	18,497	35.48	
afle		US 61 Westbound	31,129	22,672	-8,457	27.17	
9 ntr:	West	US 190	21,483	130	-21,353	-99.39	-1.38
6 -3 Coi		Westbound					
10 19	East	I-59 Northbound	27,497	33,997	6,500	23.64	
rin	North	I-55 Northbound	66,564	65,563	-1,001	-1.50	
Du	South	US 90 Southbound	40,120	42,628	2,508	6.25	
M		I-10 Westbound	4,907	14,561	9,654	196.74	
flo		US 61 Westbound	4,193	8,588	4,395	104.82	
7 tra	West	US 190	5,301	30	-5,271	-99.43	24.10
9-4 01		Westbound					
33 L C	East	I-59 Northbound	10,092	9,425	-667	-6.61	
fteı	North	I-55 Northbound	14,292	18,699	4,407	30.84	
A	South	US 90 Southbound	14,322	14,602	280	-1.96	
Total			352,369	357,406	5,037	1.43	1.43

Table 3. Comparison of volumes - temporal aggregation

Table 4. Comparison of volumes - temporal and spatial aggregation

Time Interval	General Travel Direction	Observed Traffic Volume (vph)	Simulated Traffic Volume (vph)	Volume Difference (vph)	Error %	Error % by Time Increment
0	West	30,163	22,649	-7,514	-24.91	
15 Tr t	East	10,172	8,375	-1,797	-17.67]
C ii o	North	10,991	14,354	3,363	-30.60	-7.38
Р	South	9,005	10,948	1,493	16.58	
50	West	104,750	93,437	-11,313	-10.80	
15 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	East	27,497	33,997	6,500	23.64	
C 11-	North	66,564	65,563	-1,001	-1.50	-1.38
Ι	South	40,120	42,628	2,508	6.25	
	West	14,401	23,179	8,778	60.95	
47 fer F	East	10,092	9,425	-667	-6.61	
Af C, Af	North	14,292	18,699	4,407	30.84	24.10
	South	14,322	14,602	280	-1.96	
Total		352,369	357,406	5,037	1.43	1.43

METHODOLOGY

Model Development

The simulation involved over 300,000 vehicles moving within a road network that covered several

thousand square miles and temporally during a 48 hour period during which output statistics were generated on a second-by-second basis. Figure 2 shows a schematic diagram summarizing the general flow of the coding methodology that translated the assumed Katrina evacuation characteristics into TRANSIMS model. The first step in the process required the creation of the model *Highway Network* of the region including its key characteristics (speed, number of lanes, control,... etc.). This network also served as an input to spatially

distribute the synthetic population. The second step of the development process involved the creation of a representative population of people and households in the study area using the TRANSIMS *Population Synthesizer* module. The synthetic population was based on the 2000 UScensus aggregated data and the

Time Interval (hr)	General Travel Direction	Observed Traffic Volume (vph)	Simulated Traffic Volume (vph)	Volume Difference (vph)	Error %	Error % by Time Increment
	West	149,314	139,265	-10,049	-6.73	
0-47	East	47,761	51,797	4,036	8.45	
	North	91,847	98,616	6,769	7.37	1.43
	South	63,447	67,728	4,281	6.75	
Total		352,369	357,406	5,037	1.43	

Table 5.	Comparison	of volumes -	cumulative	directional	aggregation
rabic 5.	Comparison	or volumes –	cumulative	un ccuonai	aggregation

disaggregated data from Public Use Microdata Samples (PUMS). Land use data were also used to locate households relative to the transportation networks. The synthetic population and the household activity survey files were used to feed the TRANSIMS Activity Generator module. The Activity Generator assigned travel activity patterns to individual household members and distributed these activities to locations and modes. The synthetic activity served as input to the TRANSIMS Router/Route Planner module to generate travel plans for evacuation trips. Finally, all of the movements and their interactions within the network were generated by the TRANSIMS Microsimulator module using the travel plans generated by the Router. Feedback is applied to the equilibration process iterating between Route Planner and Traffic Microsimulator. Through feedback module, some routes may be found infeasible. These activities are then passed back to the Activity Generator to determine appropriate alternatives. Some trip plans cannot be followed in the Traffic Microsimulator because of time-dependent road closures and other causes. In this case, individuals with those plans are passed back to the Route Planner for new routing suggestions. Finally, TRANSIMS can create aggregate results comparable to

traditional analysis tools. The *Microsimulation* can lead to highly detailed snapshot data; for example, the exact location of every traveler at any given time.

Data Sources

The validation process of the TRANSIMS New Orleans hurricane evacuation model was based on traffic volume data. While it has been suggested that other validation measures of effectiveness (MOE), like vehicle speeds, headways, occupancies,... etc., could have also been used to evaluate the model results, none of these parameters were available at the time of the evacuation. As a result, the basic goal of the validation was to have the modeled traffic patterns reproduce traffic patterns similar to those observed during the Katrina evacuation of 2005.

The traffic volume data used for this study were collected by the Louisiana Department of Transportation and Development (LA DOTD) Office of Planning and Programming as part of their statewide traffic data collection program. The objective of this program is to continuously record traffic volumes to monitor long-term traffic trends on a statewide level. The data are used primarily for aggregate-level planning and trend analyses. However, they can also be extracted more frequently and compiled for the assessment of traffic conditions associated with

particular events, such as, in this case, the evacuation for hurricane Katrina.



Figure 4: Regression comparison of aggregated traffic volumes

As part of the LA DOTD monitoring program, traffic volumes are collected on a routine basis using a network of 82 permanent count stations located on various roads across the state. These automated recorders are arranged to provide a representative sample of traffic on all road classifications (freeway, arterial, collector,... etc.) across the urbanized and non-urbanized regions of the state. During August 2005, 67 of the 82 LA DOTD data recorders were in operation, of these, 16 of the stations were located on Interstate (I) Freeways, 22 were on US Highways and the remaining 29 were on Louisiana State Highway (LA) system roads.

For this study, data from a total of eight stations

located on the major outbound evacuation routes from the New Orleans Metropolitan Areas were used for comparison. The approximate locations of these stations are illustrated in Figure 3. These stations were selected because they were the stations that monitored output routes in the New Orleans area while limiting the potential inclusion of local (i.e., non-evacuation specific) traffic. Several of them were also located near, or in the case of Station 42 – directly on, the contra flow segments.

The LA DOTD data used for the validation encompassed a 48 hour period from 12:00 am Saturday, August 27th through 12:00 am Monday, August 29th, reflecting the Katrina evacuation process. During this period, the hourly traffic volumes fluctuated at various times. However, the cumulative volume trend, aggregated for all stations, resulted in the characteristic Double-S cumulative distribution curve. The observed traffic volumes are shown in tabular form in Table 1.



Figure 5: Regression comparison of the northbound observed traffic volume versus simulated traffic volumes

Validation Procedure

The goal of the calibration and validation process was to ensure that the TRANSIMS generated traffic volumes were similar to those observed on the field during the hurricane Katrina evacuation. The validation procedure used in this project followed a multi-step iterative process. The initial part of this process typically involved the execution of the TRANSIMS Router about ten times. After this routine, the next part of the process involved a series of combined Router-Microsimulator runs to reach convergence. Typically, about ten of these combined trials were required because it was necessary to route the trips assessing the outcomes of this routing arrangement to determine if the traffic distribution was reflective of a realistic condition. After this lengthy process of model execution and adjustment, a final set of model volumes was produced. These data are shown in Table 2. Similar to Table 1, the volume data were generated on an hourly basis at each station during the 48 hour evacuation period. Conveniently, this arrangement permitted a one-to-one hourly comparison of traffic volume at each station permitting validation to be undertaken on both spatial and temporal bases.

VALIDATION RESULTS

The process of calibration and validation of regional simulation models is crucial to ensure that the model is realistically representative of actual traffic. The results of the validation process are presented in the following sections based on the series of analyses conducted. The first results were based on direct comparisons of the hourly volumes at each station. Based on these results, further analyses were conducted using the same data set, after grouping the data into various sets based on selected time periods or travel directions. The results are presented based on both direct computations of percentage error as well as regression analysis.

First Approach: Statistical Validation Based on Percentage Error

Temporally Aggregated Analyses

Comparison of the observed and simulated traffic volumes at each of the station locations was carried out using volumes aggregated by the time periods. In this analysis, the time periods were aggregated based on the three segments of the evacuation process. The first included the initial 15 hours of the evacuation prior to the implementation of contra flow. This spanned the period from midnight on Saturday the 27th (hour zero) to 4:00 pm on Saturday afternoon (hour 15). The second period included 24 hours of contra flow operation between 4:00 pm on Saturday afternoon (hour 16) to 4:00 pm the following Sunday afternoon (hour 39). The third period included the last 8 hours of the evacuation from 4:00 pm on Sunday afternoon until midnight after the termination of contra flow and as evacuation volume ebbed to a trickle as travel conditions deteriorated.

The results of the period aggregation analyses are shown in Table 3. In the table, it can be seen that, similar to the disaggregate analysis, the error percentages at the individual data stations are quite substantial; ranging from nearly 200 percent along westbound I-10 to less than two percent at some of the north and southbound station locations. As also evidenced in the table, a persistent discrepancy occurred in the assignment of traffic along the parallel routes of I-10 and US-61 that carried westbound traffic out of New Orleans. The issue was related to a condition in which the TRASNIMS Router tended to overutilize I-10 and underutilize US-61. This condition was particularly noteworthy during the periods without contra flow.

In an attempt to correct this problem, the link freeflow speeds on these routes were modified to achieve an assignment closer to the observed values. Unfortunately, this had only a minor effect on the numbers. Adjustments were also made to the functional classification preference setting but, once again, this also had a very limited effect on the assignment disparities between the two routes.

As would be expected, Table 3 also shows that when volume comparisons were performed on more aggregate bases, the level of error was reduced. In the table, this is most notable in the right-most column values where the error percentage decreased to less than ten percent when volumes were combined over the first two phases of the evacuation. The error percentage shows an error of just over 24 percent during the last eight hours of the evacuation after contra flow operations were terminated. It can be argued that the majority of this error is based largely on the discrepancies observed at the westbound data stations.

The side-by-side comparison of Table 3 also shows that TRANSIMS overpredicted the total number of trips out of the New Orleans area by 5,037 trip or 1.43 percent. This number is potentially significant for several reasons. First, it suggests the tremendous predictive accuracy that has resulted from the TRANSIMS *Population Synthesizer*. Results within two percent of the observed values are well below the initial expectations of the system. This discrepancy is also well below those of prior modeling systems that have been applied for evacuation simulation purposes. In recent studies (Brockfeld et al., 2004; Chiu et el., 2008), it has been suggested that error percentages between modeled and actual/observed volumes in the

range of 15 to 25 percent are acceptable for this type of modeling.



Figure 6: Regression comparison of the southbound observed traffic volume versus simulated traffic volumes

Further, this modest error value becomes more interesting when it is recognized that there were other factors that likely complicated any potential straightforward evacuation. First, it is well recognized that a percentage of the New Orleans population did not evacuate for the storm. Although no one precisely knows what this number is, it has been widely suggested to have been as high as 100,000 people or 7.7 percent of the regional population. Given typical evacuation vehicle occupancy rates of 2.2 to 2.5 persons per vehicle, this could reflect approximately 40,000 vehicles. Traffic observed at the LA DOTD count stations is also recognized to include locallygenerated non-evacuation traffic. Since there is no present method to distinguish these specific vehicle groups from the overall counts, there is no way to consider or determine the effect of these vehicles on the validation process.

Spatially Aggregated Analyses

After reviewing the results of the prior assessments, the data were further aggregated to evaluate the conditions more specifically associated with the directional distribution of evacuation traffic. To accomplish this task, the preceding temporal aggregations were further grouped by the general direction of travel of the evacuees. As shown in Table 4, the aggregation of the eastbound, northbound and southbound evacuees was effectively moot because there was only a single station in each of these directions. However, the westbound aggregation, which combined three stations, resulted in significantly lowered errors. This decrease in error percentage is quite logical and should be expected based on error percentage calculation, since some of the stations

overpredicted the demand and others underpredicted it leading to an overall "cancellation effect" between the two extremes. Another expected phenomenon illustrated in the table was the consistent cumulative error percentages for the sums across the pre-, during and post-contra flow time periods. Since the cumulative numbers did not change between the two tables, it should follow that they would be consistent.



Figure 7: Regression comparison of the eastbound observed traffic volume versus simulated traffic volumes

The results of the Table 4 aggregations suggest that while the TRANSIMS Router module seemingly experienced difficulty in replicating the route choices of the evacuees during the process, it was able to demonstrate an improvement in terms of assigning the trips toward a particular direction given the various routes. This concept was tested as further examined in the final set of aggregation groupings. In the final set of aggregation groupings, each of the directional volume sets was summed over the entire 48 hour evacuation period. The results of this analysis are shown in Table 5. The error percentages here show that at this level of temporal aggregation, all of the directional assignment error percentages were less than ten percent. This was quite encouraging from the stand point of the objectives of the validation. However, because of the enormous number of variables involved in the 2005 New Orleans evacuation, it is also difficult to completely attribute positive quantitative results purely to the quality of the TRANSIMS system or to accurate data sets, assumptions and/or analytical methodologies.



Figure 8: Regression comparison of the westbound observed traffic volume versus simulated traffic volumes

Second Approach: Statistical Validation Based on Regression Analysis

A different family of statistical tests for the validation of traffic simulation models is the regression analysis. According to Barcelo and Casas (2003), this method has been used to statistically compare the output from the simulation and the observed data for microscopic traffic simulation models in a situation in which only aggregated values are available (Flow counts at different stations aggregated to the hour). In this case, the observed traffic volumes are considered as the original data and the simulated traffic volumes are considered as a prediction of the observed data.

Regression analysis was conducted for the total traffic and then for each evacuation direction separately. The performance of the model is evaluated based on the R-squared (R^2) value of the regression line $y = a + bx + \varepsilon$. R^2 indicates how closely traffic volumes prediction matched the observed data. Its value lies between 0 and 1. The higher the R-squared value, the better the performance of the model. Although higher values are desirable, smaller values are considered reasonable considering the large scale of our model. A regression analysis was performed on the cumulative volumes as well as the hourly volumes between observed and simulated data.

Figures 4 through 8 show the regression comparisons between cumulative volumes observed in the field and cumulative simulated volumes over 48 hours for the total aggregate traffic volumes in the study area, northbound (station 15 located on I-55), southbound (station 88 located on US-90), eastbound (station 67 located on eastbound I-10) and westbound (station 54 on I-10 in LaPlace, station 27 on US-61 in LaPlace and station 18 on US 190), respectively.

The regression analyses at these locations were found to have acceptable fits with R-squared values greater than 0.80. This indicated that the TRANSIMS simulation model was a valid model that was able to realistically replicate the traffic patterns observed during the evacuation of hurricane Katrina, except for the westbound traffic volume,in which it is obvious that TRANSIMS underestimated traffic volumes due to the fact that the DOTD counts included the through traffic from other neighboring states.

SUMMARY AND CONCLUSION

This paper proposed a methodological pattern to validate regional level microscopic traffic simulation models. This study is one of the first to utilize actual field observed evacuation data. Model validation was based upon a comparison of the TRANSIMS generated traffic volumes to the corresponding traffic volumes actually observed during the 2005 hurricane Katrina evacuation. Analysis was performed utilizing percent errors estimation based on direct comparisons of the hourly volumes at each station. Further analyses were conducted using the same data sets by grouping the data into various sets based on selected time periods or travel directions. Also, an alternative validation approach was carried out using regression analysis between the cumulative observed and simulated volumes for six stations by analyzing the fit for the regression line $y=a + bx + \varepsilon$. The error percentage and the fit were found to be reasonable with an error percentage less than 25 percent and an R-squared value of over 0.80. This indicated that the TRANSIMS simulation model was a realistic representation of the

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